



Optimal Timing of Preventive Maintenance for Addressing Environmental Aging

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- Background
- Needs
- Needs being addressed
- Needs Not being addressed well enough
- Combining efforts nationally
- Suggested emphasis of this PFS





THE PROBLEM



- Binder oxidation and hardening DO occur extensively beyond one inch down into the pavement
- Mixture performance declines significantly with binder oxidation
- Effective maintenance programs will inhibit binder oxidation in pavement or rejuvenate inplace binder Is this possible?









IN SERVICE: BINDERS OXIDIZE, BECOME STIFFER AND LESS DUCTILE...A RELENTLESS PROCESS!









AS BINDERS OXIDIZED, MIXTURE FATIGUE RESISTANCE DECLINES...







BINDER OXIDATION MODEL CAN BE USED TO ESTIMATE HARDENING RATE IN PAVEMENT







THROUGHOUT SERVICE, BINDER HARDENING PROCEEDS IN A WAY THAT DEPENDS DEPENDS ON CLIMATE AND THE PHYSICAL STRUCTURE OF THE MIXTURE









ACCESSIBLE AIR VOIDS IS ONE OF THE KEY MIXTURE PARAMETERS THAT SIGNIFICANTLY AFFECTS BINDER OXIDATION

















- Effects of fog seals on pavement durability appear to be minimal, with respect to sealing or rejuvenation
- Fog seals did not appear to penetrate below the pavement surface
- The aging rates of asphalt binders are decreased by very low accessible air voids









- Binder oxidation and hardening DO occur extensively beyond one inch down into the pavement
- Mixture performance declines significantly with binder oxidation
- Effective maintenance program would inhibit binder oxidation in pavement and/or rejuvenate in-place binder
- Evidence suggests that sealants may affect binders...or may not





RESEARCH NEEDS



- Improved Understanding of binder oxidation and hardening rates in pavements (model)
 - Improved measurements of mixture air voids morphology: pore size, spacing, AAV
 - Improved understanding of air permeation through pavements
- Improved understanding of the impact of binder hardening on mixture performance (e.g. fatigue)
- Field measurements of binder oxidation in pavements and maintenance treatment



effectiveness



CURRENT EFFORTS



- Improved Understanding of binder oxidation and hardening rates in pavements (model)
 - Thermal/Oxygen transport model ARC, 0-6009
 - Improved measurements of mixture air voids morphology: pore size, spacing, AAV - ARC
 - Improved understanding of air permeation through pavements - ??
- Improved understanding of the impact of binder hardening on mixture performance (e.g. fatigue) -0-6009 (laboratory, field data, Texas mixtures); ARC (modeling, laboratory, field data, non-Texas)
- Field measurements of binder oxidation in



pavements and maintenance treatment effectiveness - 0-6009 (Texas)







TxDOT 0-6009: Evaluate Maintenance Treatments to Reduce Aging





- Improved Understanding of binder oxidation and hardening rates in pavements (model)
 - Improved understanding of air permeation through pavements - Pavement breathing?
 Permeation from below? Flow out the edges?
 - Do treatments restrict access to oxygen?
 Compete with moisture drainage?
- Field measurements: binder oxidation; maintenance treatment effectiveness - more data are needed in many climates to give better
 confidence in models









- Additional measurements of field aging and maintenance treatment effectiveness - flow into and through pavements; ability to retard oxidation and/or rejuvenate binders
 - Hot-applied treatments
 - Emulsion Treatments
- A better understanding of fundamentals will allow determining optimal timing - link to fundamentals of binder oxidation in 0-6009 and ARC







...Discussion...







...Backup Slides...









Model Development Approach







Equation for Oxidation Model



$$\frac{\partial P}{\partial t} = \left(\frac{\partial D_{O_2}}{\partial P}\right) \left(\frac{\partial P}{\partial r}\right)^2 + D_{O_2} \left[\frac{1}{r}\frac{\partial}{\partial r}\left(r\frac{\partial P}{\partial r}\right)\right] - \left(\frac{cRT}{h}\right)r_{CA}$$

$$\frac{dCA}{dt} = r_{CA} = AP^{\alpha}e^{-E/RT}$$

Where

Ρ

- Oxygen partial pressure in asphalt binder film =
- Order of reaction α =
- Е = Activation energy
- D_{O_2} Oxygen diffusivity in asphalt film =
- Experimental constant С
- R Gas Constant =
- Absolute temperature of asphalt film Т =
- Henry's law constant h =





TEST PLAN



Measure Field & Lab Binder Aging Rates to Calibrate the Transport Model





CMSE Test Procedures



Test	Loading Configuration, Test Parameters, and Output Data
Wilhelmy Plate- CMSE	Automatic immersion and withdrawal of binder-coated glass plates into/from liquid solvents up to approx. $5 \text{ mm} (0.2 \text{ in}) \text{ depth } (\underline{a} \ 20\pm2 \ ^{\circ}\text{C} (68\pm4 \ ^{\circ}\text{F}). \text{ Test time: } \cong 45 \text{ minutes. Measurable & output data are dynamic contact angle } (\theta) \text{ and surface energy (SE) components for the binder } (\Gamma_{i\text{-binder}})$
Universal Sorption Device- CMSE	Clean oven dried 50 g (0.1 lb) aggregate of fraction size (4.75 mm (No. 4) < aggregate size < 2.63 mm (No. 8)). Measurable parameters are vapor pressure & adsorbed gas mass of liquid solvents @ 25 ± 2 °C (77 ± 4 °F). Test time: 60 to 70 hrs. Output data is SE components for the aggregates ($\Gamma_{j-aggregate}$).
Tensile Strength (TS)- CMSE	Tensile loading until break @ 0.05 mm/min (0.002 in/min) @ 20 °C (68 °F). Test time: $\cong 5$ minutes. Output data are HMA mixture tensile strength (σ_i) and failure strain (\mathcal{E}_f).
Uniaxial Relaxation Modulus (RM)- CMSE	Trapezoidal shaped strain-controlled @ 200 microstrain (tension & compression), 60 s loading & 600 s rest period @ 10 °C (50 °F), 20 °C (68 °F), and 30 °C (86 °F). Test time: \cong 25 minutes. Output data are HMA mixture elastic relaxation modulus (E_i), stress relaxation rate (m), and temperature correction factors (a_T).
Uniaxial Repeated Direct- Tension (RDT)- CMSE	Image: Image in the strain control is the strain control in the strain control in the strain control is the strain control in the strain control in the strain control is the strain control in the strain control in the strain control is the strain control in the strain control in the strain control is the strain control in the strain control in the strain control is the strain control in the strain control in the strain control is the strain control in the strain control in the strain control is the strain control in the strain











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BACKUP SLIDE 2



Improvement over EICM and recent advanced models

Improvements Over EICM and existing Models			
	Our Model		
Surface B.C	$\rho C \frac{\Delta x}{2} \frac{\partial T_s}{\partial t} = (1 - \alpha)q_s + \varepsilon_a \sigma T_a^4 - \varepsilon \sigma T_s^4 - h_c (T_s - T_a) + k \frac{\partial T_s}{\partial x}$ q _s [shortwave solar radiation] $\epsilon_a \sigma T_a^4 \text{ [incoming longwave radiation]}$ $\epsilon \sigma T_s^4 \text{ [outgoing longwave radiation]}$ h _c (T _a -T _s) [convection heat loss]		
Heat conduction inside pavement	$\frac{\partial T}{\partial t} = \frac{k}{\rho C} \frac{\partial^2 T}{\partial x^2}$		
Bottom B.C	Depth independent heat flux 🛛 🔸 📩		
Input data			
Та	Interpolated hourly air temperature with ** max. and min. temperature recorded		
Qs	Hourly solar radiation predicted using SUNY** or METSTAT model (available at NSRDB)		
Wind speed	Hourly wind speed		
Model Parameters	Ontimized model perometers (Record er		
wouer Parameters	temp. measured in the middle depth of Pav.)		



* Improvement over EICM * over recent advanced models